A Survey Study on Retrofitting of RCC Building with Beam Design Criterion

¹Manish Priyadarshi, ²Abhishek Sharma

¹M. Tech. Scholar, ²Assistant Professor Civil Engineering Department Structural Engineering CBS Group of Institutions

Abstract: Multi-story buildings are being constructed in India as a result of the high costs and scarcity of available land. To make the most of the most extensive and available land area, high-rise building design arrangements are utilized, especially in regions with the largest amount of available land. These buildings may be harmed if they are located in an earthquake-prone area. earthquakes are a natural occurrence that have the potential to cause the most significant damage to a structure. In order to have a flexible failure form, the structure must be safe, which means it must be designed correctly and have the proper structural component requirements. It follows from this design concept that structures must be constructed to survive the force produced by the design earthquake with little damage, and they must also be built to withstand the extra force obtained by taking into consideration the maximum potential earthquake impact. This research focuses on the city of Delhi in the National Capital Region, which is located in seismic zone V. In this study, the results show how easy it is to provide an analytical assignment that can be used to correctly estimate the seismic resistance of current or modified systems. In order to perform damage assessments on one component of the structure, a non-linear, complicated examination of that component was carried out. One of the most significant advantages of this specific study is the use of nonlinear payment functions.

Keywords: RCC Building, Reconstruction, Beam Design Criterion

INTRODUCTION

To decrease or eliminate the danger of damage from floods, erosion, wind, earthquakes, and other hazards, retrofitting may be done to an existing building. The guidebook aims to help with floods-damaged building renovations. The following sections explain the intended audience, content, and arrangement of the guidebook.

I.

Goals and Intended Users

This guidebook has been made possible because to the efforts of various organizations like FEMA, who help local governments, engineers, architects, and property owners when preparing and carrying out residential flood restoration projects. To ensure engineers, architects, and local codes can comprehend technical feasibility and cost-effective flood rehabilitation, the purpose is to give them with engineering design and economic assistance.

This manual's purpose is to restore homes in flooded areas that are completely separate from the adjacent water wave section. This guidebook details a variety of both active and passive flood prevention mechanisms. This consists of the height of existing structures, relocation of structures, flood barriers (leaves and barriers) or liquid barriers, and various kinds of pumps for drainage systems (e.g., sealants, enclosures, sump pumps, and black flow valves) (flood protection damage-materials and protection of used contents).

The purpose of this handbook is to keep you up to date on the newest developments and organize them for easy reference. Data from previous reports and industry standards serve as the foundation for this paper. This encompasses everything from remedial measures, the concept and design of the refurbishment, and case studies of the finished project. A variety of different approaches have been put up to evaluate the various choices.

Code authorities, engineers, and architects must know that alterations to a building's construction might influence the effect of flooding on other structures in the area. To effectively handle the threat, it is essential to use a comprehensive strategy. During the rehabilitation process, effects on the water surface, including flood risks like erosion, and on flood hazards such as erosion and debris flow, as well as on non-flood hazards like earthquakes and wind, should also be addressed. The structure's capacity to endure all of the threats outlined above would be undermined if it were not designed to resist simply the flood's effects. The selection of enhanced procedures and the design process are both crucial since multi-hazards exist.

Methods of Retrofitting

The precautions in place to prevent flooding-caused catastrophes are as follows.

- Elevation: elevation: The elevated level of the existing buildings, such as perimeter walls, piers, columns, or pilings, that are on fill or foundation components such as solid.
- Relocation: Relocation: Moving buildings and other physical assets out of the floodplain.
- **Dry Flood proofing:** to increase the structural integrity of buildings while keeping them waterproof by strengthening existing foundations, floors, and walls against floods.

- Wet Flood proofing: It is possible to safeguard utilities, which make structural elements, and contents from flood during flooding of the building.
- Floodwalls/Levees: The Place has floodwalls and dams around it, as well as levees and dikes.

Passive conversion methods may be used or not, depending on whether or not human involvement is required. When an immediate or urgent remedy is implemented, results are only found if enough warning time is provided for workers and equipment to be mobilized. As a result, it is recommended that every possible effort be made to conduct passive retrofitting with no human interaction needed.

Elevation

A good retrofitting strategy is to elevate the building to keep the water away from damaged areas. Structure heights are increased above planned flood elevations to prevent the structural damage resulting from foundation flooding (DFE). Jacks used to enhance existing buildings are considered heavy-duty. Cribbing helps in the foundation-building process by supporting the structure and serving as a base. Foundations such as piers, columns, and piles are often utilized instead of new support walls. In certain cases, a building which is in place while the reservoir is being filled may also be in place afterwards.

While rising ground protects buildings against flooding, additional threats to the building must be taken into consideration before employing this technique. Walls and roofs that are elevated may create greater wind on the wall and foundation systems, and foundations that are in place already may bear greater stresses. Piers, posts, columns, and piles may be significantly influenced by seismic activity, erosion, scour, ice or debris flows, mudslides, and alluvial fan stresses, as well as movement and impact failure due to seismic activity, erosion, scour, ice, and debris flows.

Elevation on Solid Perimeter Foundation Walls

In locations with low to moderate water depth and velocity, the height of the solid perimeter foundation wall is employed. The newly raised structure is able to use materials such as concrete block units (CMU) or cast-in-place concrete for support walls if needed. Fig. 1-1 depicts a top view of the foundation wall that is made of solid material. Footings may need to be bigger if the building or the environment (such as flood, wind, earthquake, snow, etc.) poses a greater threat. In order to get the structural stability, the new steel reinforcing bars may be required to be used to create new walls.

Shallow floodwaters may be utilized to carry items, while shallow floods may help bring down a building. Use holes or vents to balance hydrostatic forces, therefore reducing the hazard. A flood protection opening is required in the event of new, considerably damaged, or upgraded structures.



Figure 1.1 Elevation on solid perimeter foundation walls

Elevation on Open Foundation Systems

A horizontal structural part that supports the structure at crucial locations using an open foundation base system that is attached to the structure using at-level attachment members. Piers, columns, and piles are examples of open foundation base systems. **Elevation on Piers**

Foundations supported by reinforced concrete or a reinforced concrete base are the most prevalent examples of open foundations. While they are common in constructions, piers are often found to be the least reliable way of elevating structures. Piers are usually built to support vertical loads in conventional applications. However, shifting floods and debris affect piers when exposed to floods. Seismic loads may cause horizontal loads as well. Retrofitting piers have to be able to carry the vertical weight of the structure, while also shielding the horizontal stresses that may be exerted on them.

Piers are often utilized for shallow flood situations, low-speed ice, debris flow, and CMU. Either situation involves using rebar on the pier and on the foundation that supports it. Every piece of reinforcement should be connected together to prevent it from being separated. If the superstructure and the pier are not properly connected, then they will not be able to endure earthquakes, wind, and buoyancy (uplift). Foundations with shallow pier foundations are vulnerable to overturning if there are significant vertical and horizontal pressures acting on them. Figure 1.2 depicts a residential construction which is supported by pilings.



Figure 1. 2 Elevation on piers

Elevation on Posts or Columns

In cases when there is a modest depth and velocity of flood, elevations on posts or columns are often employed. Generally, posts are made of wood, steel, or concrete and designed to provide simple connection to the house structure. Also, posts may be circular. Posts are normally embedded in concrete pads if they have to carry heavy loads. When it comes to placing concrete, dirt, gravel, or crushed stone into the hole and around the base of the post, it is normally backfilled into the hole and around the base of the post.



Figure 1. 3 Elevation on posts or columns

When the flood circumstances involve moderate depth and speed velocities, then it is common to elevate posts or columns. The columns are often square since this facilitates the connection to the building structure. Although, the post may also be round, the round post is preferred. Columns are often set in holes pre-drilled into concrete. To keep the column stable, you often fill the hole with concrete, dirt, earth, gravel, or crushed rock around the base of the column. The while piers is often used to support a column but may function as a separate support unit. Besides medical and recreational support devices, such as wooden knee and cross braces, steel bars, and wires, there are also varied support technologies. Technology employed depends on a number of factors, including the availability of construction materials, local flood conditions, and loads. Consider figure 3, where they have a blog article with a column basis.

II. LITERATURE REVIEW

U. Vitiello et al. (2019) A computerized depiction of the physical and functional aspects of the facility that's utilized to oversee the whole development process (design, construction, management of the entire life cycle). One of the common ways that BIM is described is as a digital depiction of a building. However, some believe that it is also a tool to allow information sharing and interoperability through the whole building life cycle. Businesses might suffer from the effect of routine maintenance jobs and unanticipated occurrences throughout the life cycle of their building. This has shown the building's vulnerability and the need for financial investment. While the cost of maintaining the facility is normally fixed, predicting additional expenses if the event is not known is difficult. It is essential that earthquake-prone structures undergo a life cycle cost (LCC) study.

Given construction costs, it is crucial to include expected losses, such as initial and post-earthquake repair expenses, in structural design. To analyze the economic performance and economic loss of buildings exposed to seismic risk, a simplified technique that is based on a semi-probabilistic approach is applied in the BIM model. Additionally, to help ensure the long-term viability of the facility, BIM processes are used to boost process viability while also carrying out vast quantities of data damage and cost analysis on the components that comprise the facility. This specific BIM method for economic loss assessment has been deployed in a manner that includes consideration of safety and economic aspects while determining overall earthquake rebuilding plans.

Mario Lucio Puppio, Martina Ferrini (2019) maged buildings and on masonry buildings in seismically active regions. Introducing an external support system is a good way to facilitate practice. A simpler version of this intervention may be done by implementing dissipative linkages. Placement, form, and length are design factors for building a link. Link support optimization, a generic approach offered by applying a set of similar external constraints, as shown in the graphic. With regards to ULS and DLS, cheaper and more efficient designs are achievable by using dissipative connections. In case of earthquakes, dissipative linkages allow for more readily interchangeable linkages.

In this paper, they propose to design a support system that maximizes energy dissipated through dissipative links. Introducing dissipative interconnections generally is costly and necessitates complex technological systems. In this article, commercial accounts are used to imitate dissipative linkages. The method described here is inexpensive and unique compared to the main proposal here. The benchmark model runs the software. The link reduction and diagonal are taken into consideration (BTA to BTD). Determine some controller settings. Considerations such as structural behavior and maximum displacement should be taken into consideration while analyzing the load-bearing components of existing structures. By using the typical intervention design, considerable cost savings may be realized. In benchmark model survey, the cost of material used is reduced by 42%.

Furthermore, the comparison of linear and nonlinear analysis provides the author with an effective tool for the structural designer, as it enables them to compare the two analyzes. It is possible to enhance the structural response of a structure even in the presence of anomalies in geometry and mechanics. One definition of strategic design parameters is that they are dissipative linkages. This intervention technique, in recognizing the system's intrinsic behavior and maximal mobility, accepts responsibility for protecting the system. A benchmark of intervention effectiveness is required for evaluating existing buildings, but this is not essential for the evaluation of interventions. They may construct a low-cost "mechanical fuse" at the interface between the building and the support system by placing the dissipative element there. This link section may be built to minimize the overall length and cross section, reducing the overall cost.

Emre Ercan et al. (2019) This article examines the CFRP panel's performance in comparison to an internally reinforced concrete beam-column junction. The percentage of work is done to investigate the behaviors of joints subjected to reverse cyclic loading in order to examine the fragility and strength of such joints. To construct an existing structure that was inadequately planned at the junction, the goal of the percent e research was to do this. Experimental analysis using external joint application. A four-step therapeutic treatment for curing included using four distinct reinforcement setups. For each connection, the arrangement was designed to highlight the many benefits of the example connection. Compared to the control sample, the changed sample's cycle performance fulfills the standards of the current Building Code. Incorporating the application of the component with the original state enhances bearing capacity and ductility.

Yasmeen Taleb Obaidat et al. (2019) This article discusses experimental work done to help repair a beam-column junction damaged by water leakage, where CFRP panels were used. This investigation was done to find out how the different rehabilitation strategies impact the lifetimes of repaired RC beam-to-column connections under cyclic loads. Eight samples were created and tested to assess the repair effectiveness for slightly damaged RC joints, specifically as it relates to flexibility, stiffness, and strength-capacity. Control samples were subjected to cyclic loading to test for failure. To ascertain the load level, the seven samples were loaded. The estimated load they can sustain is around 80% of the projected load (called preload). After mending them utilizing different CFRP board solutions, retest them to make sure everything is working correctly. Load to draft ratio, initial stiffness of the curve, ductility index, and maximum load are all used to measure the outcomes of tests. The fracture mode and CFRP deboning were monitored in addition to the strain and failure mode of the sample. Bone removal of different CFRP designs might be delayed thanks to the data, which highlight an essential problem of mending and increasing the joint's functionality. As the total of all the restored joints rose, the strength of all the beam and column joints almost approached the current shear strength of the joints. The restored joint is stronger than the one it replaces, as a consequence.

Saeid Tarfan1 et al. (2018) It suggests a new evaluation approach for buildings built with reinforced concrete and aramid fiberreinforced polymer, with a focus on evaluating the likelihood of non-ductile behavior (AFRP). Based on ancient construction traditions, RC structures of three different heights (4, 6, and 8) were built. Then, all of the structures' highly detailed columns were strengthened using pre-tensioned AFRP belts. A finite element model of the Open Plastics hinge was built utilizing the original column's shear weakness and the beam's decreased stiffness and strength. Using global and component level measurements, quantify structural performance incrementally by using incremental dynamic and nonlinear static analysis. Vulnerability curves, yearly average collapse frequency, and collapse margin ratios are used to calculate global vulnerability responses. Additionally, statistical analysis was conducted to gain knowledge on the horizontal movement of the inter-layer slippage, the physical properties of structural components, and the amount of energy that was lost during three distinct degrees of seismic danger. Pre-tensioned AFRP modification leads in improvements in overall ductility as well as reduced collapse chance. Moreover, increasing the number of tales in the crash mechanism helps to keep tales from forming. Component-level pretension AFRP enhances the ductility and dissipated energy of the column, which, along with the fact that it enhances the energy capacity, particularly at the crash limit, results in an improved performance in general.

Cong-Thuat Dang and Ngoc-Hieu Dinh (2017) Reinforced concrete beam-to-column connections in historical structures are under discussion, with several different ways offered. Four semi-scale RC outer beam-column joints were tested in a manner simulating seismic activity to see whether they can support a load. The control samples were created to demonstrate the presence of joint shear. To make sure that the proposed retrofit technique would work for the real structure, the proposed retrofit techniques were tested on control specimens, taking into consideration many architectural factors, including the building's steel façade and lumbar corrective solutions.

Damage and load drift, ductility, dissipated energy, and strain distribution of longitudinal bars were evaluated based on structural characteristics such as damage and damage, load drift, ductility, and dissipated energy. The experimental findings reveal that the suggested solution reduces the panel area shear deformation and hence strength, deformability, and energy dissipation ability.

Matsutaro Seki et al. (2016) Tokyo Rikkyo University in Japan built a new church utilizing basic seismic isolation technologies, utilizing the existing structure as a foundation. This is the first rehabilitation project applied to antique Japanese masonry. The

seismic index (Is) is low, and as a result, it is going to be retrofitted. Several concepts have been put forward, and eventually the use of fundamental isolation methods was used to keep the building's inside and outside in mind. Before creating a restoration procedure, load testing and field surveys are conducted to determine the masonry's strength and decay. The results of the non-linear seismic isolation response simulation were then followed by the determination of the fundamental seismic isolation system.

Finally, the most critical building construction required throughout the refurbishment has been accomplished. After verifying many test results, such as design measurements, microvibration measurements, human body vibration testing, and load testing, numerous further verification tests were done. Currently, a very strong seismic monitoring system is in use.

They will now look at the transformations undergone by several historical stone buildings by the use of fundamental seismic isolation measures. The established improvement approach of basic seismic isolation has shown that buildings' weaknesses and architectural elements may be addressed with it. The ground-floor and ground-level accelerometers are both in use.

Fabio Mazza et al. (2015) According to the design technique developed for proportional hysteresis damping support (HYDB), which uses displacement to simulate real-world conditions, structures built with reinforced concrete (rc) and with an irregular construction are required to meet certain performance specifications. set up To test the design procedure's validity and reliability, numerical survey was done on the 6th floor room check. When the previous Italian earthquake code (1996) applied, the building was built as a medium risk zone; since then, it must be retooled to attain the high risk capabilities of the new Italian code (NTC 08). Additionally, brick walls are scattered throughout the borders to mimic vertical imperfections. window glass will be the replacement. Non-linear dynamic analysis of unfilled and filled framework systems, together with real and simulated ground movements, response spectra for different performance levels, is done for all frameworks, including those that have already been filled or unfilled.

The response spectrum is appropriate. In order to do this, a bilinear moment-curvature law is idealized, assuming a two-component model, where the bending moment relies on the axial load. While the HYDB reaction also passes the hyperbolic law to avoid buckling, it does so in a somewhat different manner. Lastly, masonry fills are only elastic while under compression, and they are depicted as struts that go through constant diagonal movements with an elastic-brittle linear relationship.

Hybrid design of home-to-work shifts has been suggested for the use of DBD process for the seismic analysis of irregular frame structures owing to variations in home-to-work use. For this reason, the two-story high building's initial two floors were made of reinforced concrete instead of stones. Two structural solutions are generated from the fill frame by employing the proportional stiffness and strength (DBIF R) and constant drift and shear ratio (DBIF IR) design requirements of the residential building HYDB (ie IF). Assumption: It takes tmin = time required for the two structures to go through their simulation, after which the response of the IF structure is equivalent to that of the frame which is unsupported (ie UF).

However, when it comes to maximum final time (tmax), UF structures have greater performance than IF structures, although the entire simulation length is same. Breathing room for beams and columns equivalent to DBIF R was found with respect to tmin, but not considering tmax, DBIF IR structures are more efficient. While life safety is a concern, the building is also appropriate for damage or collapse. Almost uniform rather than design values were found in the DBIF R structure, whereas an average level of variability seen just above design values was discovered in the DBIF IR structure.

S Karthik and Karthik Sundaravadivelu (2017) It is thought that the focus of the research in civil engineering should be to reinforce damaged structures. The change's objective is to reestablish the original structure of the components. The main goal of the project was to research the effects of reactive powder concrete (RPC) overlays on reinforced concrete beams. At 1200 mm in length, width of 100 mm, and depth of 200 mm, these reinforced concrete beams were driven into M30 grade concrete and were left to cure for 28 days in the laboratory. To determine the ultimate load, the beam was utilized as a control and then tested with two point loads. The last 90% of the beams have been subjected to final load control. Most of the beams have been affected by something, but to minimize the amount of damage to the beams, those damaged beams are first repaired with a thick reactive powder concrete overlay on the tensile surface of the beam, and the side cover layer is built on corrective techniques using layers of 10 mm and 20 mm thickness. It had been optimized. Structural homogeneity is improved by adding steel fibers to reduce the number of coarse particles in the mixture. The findings of the load analysis for the modified beam were compared to the findings for the control beam.

The experiments demonstrate that reactive powder concrete may be used as a modifier, and it is confirmed that fresh concrete is bonded using Nitobond EP epoxy glue. The findings reveal that exfoliation does not occur in the experiment. They have received the following findings.

Once stress is increased, the modified beam may resume its original strength and bends the whole structure. An overlay beam with a ten millimeter (mm) thick face exerts a maximum load equal to that of a control beam, whereas an overlay beam with a thickness of twenty millimeters (mm) on the tension side imposes an 8.6% increase in load.

As compared to the control sample, the stresses applied to 10-mm-thick RPCs rose by 10.2% and the stresses applied to 20-mm-thick RPCs rose by 22.9%. An increase of 4.1% and 13.5% was seen for beams with improved RPC thickness of 10 mm and 20

mm, respectively, compared to the control samples, while the stitching approach raised loads by 11.5% and 12.3% for beams with RPC thickness of 15 mm and 20 mm, respectively. Delaminated concrete with a 20 mm RPC overlay grid shows significantly increased load bearing ability in comparison to beams treated alone with RPCs. While mesh techniques could be less expensive, as compared to the other two ways, in terms of cost, grooving and stitching might be more inexpensive. The RPCs may be overlaid using all three improvements to strengthen the beam. Bending does not lead to any damage in the structural part, since no peeling occurs.

Paolo Foraboschi (2016This study illustrates how structural improvements to a school building, without considering the effects of an earthquake, don't ensure that the structure would fall under gravity load. Steel offers numerous shapes and functions for cold-formed components, thin-walled profiles, welded components, and steels. used to Numerical computations are not used as a method of structural design in the activities given here. Due to the many uses for steel, the steel structure is especially suited for construction of reinforced concrete structures in the twentieth century, in particular.

C.Z. Chrysostomou et al. (2016) Seismic reconstruction of RC frame structures by changing chosen bays into new walls by filling RC walls was experimentally explored using a full-size four-layer model which was evaluated using the pseudo-dynamic (PsD) approach. The frame is developed and developed for gravity loads using simply the varied connection details between the wall and the border frame. Two separate numerical models were created in order to replicate the experimental reaction. The primary goal of this work is to demonstrate the applicability of these models in simulating the nonlinear behavior of RC structures with RC Encrypted Walls, and to address their veracity.

In the second part of this research, they derived the vulnerability curve of the refurbished building under a particular refurbishing procedure by doing a vulnerability analysis of the reinforcing framework. Experimental data that demonstrate the spread of damage along the mixed vulnerability curve may be correlated to the floor displacement experiment. To examine the brittleness of the retrofit frame, the researchers constructed a model using a basic analytical equation and utilized it to develop a brittleness curve. When compared with a pre-conversion vulnerability curve, the current vulnerability curve reveals critical quantitative information about the degree of catastrophic damage and structural collapse. retrofitting the RC building by inserting the RC wall was also included into the framework, and the performance of the new building's dynamic characteristics was improved, too.

The main goal of this work is to investigate the possibility of simulating experimental nonlinear behavior of a four-story RC structure with walls reinforced with RC-filled concrete. Simple arithmetic is to be used. Estimation of a person's level of vulnerability These analyses compare the findings of two models with actual testing of a full-scale RC construction which were performed using pseudo-dynamic loads. For a better comparison, use floor displacement and basic shear time history. Frequency value comparison reveals that the building's physical attributes are well approximated. Both models were able to reproduce the tested building's actual behavior and exhibited good overall performance for all response parameters. It can be deduced that employing particular joint details will result in an overall behavior, since the interface between the RC filling wall and the bounding frame in the numerical model does not enable relative motion.

Muhammad N. S Hadi &Tung M. Tran (2016) The joint shear strength model for the reinforced joint was created using this study's findings. Four RC external connections were designed and constructed with no lateral reinforcement. Reverse cycle loading was used to verify the strength of the design. In order to compare the results of the control joint to the other three jointed joints, they were taped to the concrete coverings of the columns around the joint region, which were modified from square to circular cross sections, and wrapped with various quantities of CFRP. This design improves the overall strength of the joint by strengthening it using mean plane stress and employing 32 connections, three of which are reinforced using traditional FRP. strategy The method is tested by making use of databases that include network connections. This research has shown that altering the percentage of CFRP in the vicinity of the specimen leads to varied failure modes for the linked connection, with the exception of shear and seismic performance; yet, the connection's shear and seismic qualities are enhanced. The task is finished. With the new technology, deboning is no longer required due to concrete surfaces or the poor confinement effect of FRP, resulting in substantial advantages. In this case, the experimental and analytical findings demonstrate that the novel reinforcing approach does, in fact, help enhance resistance to FRP/concrete joint shear. The proposed model predicts the composite shear strength of FRP reinforced joints with a fair degree of accuracy. It is envisaged that the suggested approach and model would be adopted in real-world applications because of its exceptional performance.

Hussein Bark et al. (2016) A number of measures have been put in place to help deal with the extensive damage that earthquakes have caused to buildings. Some of the most often employed procedures to strengthen the structural capability of a building before it failed were found to be insufficient for its seismic-stability requirements. Engineers who have a reliable objective and logical technique to supply a tool to assess the final design come into an issue because of reinforced concrete constructions. The goal of this research is to find a way to overcome this inherent constraint by carrying out a rigorous, holistic investigation of reinforced concrete shear walls with three-dimensional solid finite components, which are reinforced concrete buildings five stories tall. To polish cracks, you use the smear crack approach, and inserted rod parts resemble steel rods. The five-story, restored RC building received a push analysis using this thorough modeling techniques for the first time. Comparative numerical findings reveal that the suggested evaluation approach is correct computationally. It is essential to further lower the computing needs while processing full-scale models by using parallel processing. These investigations forecast numerical investigations of the improved RC structure's mechanical behavior, employing 3D accurate analysis. In order to better understand the potential issues, an entire five-story RC

building model was built before and after renovation, and a single shear wall component was utilized to reinforce the building's structural behavior (filling the RC wall). Additionally, the impact of the single shear wall component on both seismic loading and the Behavior of the building were investigated. All structures have been responded to, and their overall influence has been addressed. Analysis of numerical findings has shown that increasing the shear wall's reinforcing design greatly boosts the structure's seismic resistance. One shear wall can take all the seismic load and the additional impact resistance because of retrofit. This amounts to 333% increase in structural strength. After retrofitting an existing frame system, a parametric research discovered that reinforcing shear walls in a new way specified the right implementation of the reinforcement element and the significance of ensuring that the reinforcement is connected to the existing frame. Appropriate reinforcement sizing in the existing frame system is critical to overall shear wall shear capacity. A smaller shear wall shear capacity might result in a less expensive implementation.

Additionally, the Eurocode calculation underestimated shear wall support by 20 percent. Even though the building has a horizontal symmetry, the intricacy of the resulting reinforced shear wall causes complicated deformation patterns to arise due to the irregular forms and varied materials used (bending and twisting in 3D) promotes In light of this suggested method, enhanced guidelines and design will become both feasible and cost-effective.

Using the whole model to find out how much seismic force it can handle also indicates that the development procedure is resilient when dealing with estimated values of 500,000 degrees of freedom. you are an artist Computational efficiency is needed to accomplish continuous solver tasks, as well as parallel solver tasks, as well as interface member effect across the current concrete and the new concrete. Future research is planned on the rebuilding of concrete. In the future, the objective is to do a thorough five-tiered RC construction cycle analysis.

Antonio Formisano & Federico M. Mazzolani (2015) this paper's techniques on building seismic reconstruction by using three separate multi-criteria choice approaches and on ultra-high item selection of existing masonry structures.

The software application has two case studies in which it is referenced. The first intervention on a 3D RC structure that has been changed using various seismic devices mostly based on metal elements is conducted on a true-to-life life-size replica. Before the project begins, a trial is first carried out to measure the results, absolutely It turns out that all MCDM approaches result in the same outcome. Aluminum shear plates play an important part in the seismic analysis structure's seismic reconstruction.

Alternatively, the construction techniques investigated to expand the number of floors in existing masonry structures use multiple technologies, including building systems as well as improvements in technology. The results of these treatments have been shown to be beneficial in increasing building compliance in southern Italy. The research found that utilizing the three MCDM approaches yields the lightest, most economical, and most sustainable solution for cold-formed steel structures. In this article, three MCDM approaches were used to evaluate the efficacy of two structural modification interventions, one of which was earthquake rebuilding and the other of which was the installation of additional floors. DM's primary parameters are derived from the following three comparison variables: structural, economic, and environmental measurements. This research brings something new to the table by offering two case studies to explain the answer to a fair MCDM issue utilizing several survey methodologies, allowing the user to personalize their approach.

The first step was based on real-life 3D RC structures which had previously undergone various testing activities employing several 3D seismic reconstruction devices. The same results are obtained by all the analytic techniques, including the MCDM, which corroborates the key function of the aluminum shear plate. Additionally, when the same three MCDM approaches were applied to more thorough research, the same findings were found. in these instances, as is the case with the various weights, lead shear plates provide the best combination of results, which results in a global ranking. As a result, it has been shown that metal equipment dominates the current use of RC restoration frames for seismic activity. It is important to distinguish between intervention 1, in which masonry building units were erected vertically using conventional and new technology, and intervention 2, in which existing masonry building units were vertically added by the use of conventional and new technology. When it comes to cold-formed steel systems, the three different procedures that provide the same results all have the same prerequisites, and these are: low weight, economy, and sustainability. Three of the conventional weight change weight changes have a minor influence on the answer, and the solution is shown to be reliable since three of the robust weight change tests show it to be correct. In conclusion, practitioners who are doing structural alteration interventions are more likely to use retrofit and vertical addition for economic, structural, and environmental purposes.

Antonio Formisano & Dipti Ranjan Sahoo (2015) One of the key economic and social activities being pursued is research on the seismic retrofit of existing reinforced concrete structures. Metal equipment with a new innovative reversible technology is used to give a high degree of structural safety while also protecting the structure from harm. In this article, Steel Plate Sheer Wall (SPSWs) was customized for high rise residential RC structures situated in the historical heart of Torre del Greco (Napoli, Italy) which are capable of vertical loading. I applied it as I did. The structures being inspected showed evidence that they were not strong enough to survive the impacts of the earthquake. Through partial compartments and full compartment SPSWs, the retrofit design of such a building has been accomplished. Non-linear analysis was used to examine the behavior of the restored building. Structural qualities such as strength and stiffness are enhanced when the tested buildings are upgraded. As further research and development is completed, more information becomes available that will aid in the development of improved SPSW designs for existing RC buildings, which will allow for the development of more useful designs that target the design of optimal panel shape to meet certain performance goals.

This research offers a design strategy for increasing the strength of multi-story RC constructions subjected to seismic tremors using SPSW. The first evaluation by the first author consisted of a two-dimensional, sheet metal-reinforced three-dimensional concrete

frame and was implemented and expanded to many multi-story structures in Torre del Greco, located in the city center. (inside the Napoli region) The methodology suggested here is used to get dimensions for the two panel types (partial and full parentheses).

The study found design predictions which demonstrate an increase in building performance, in terms of stiffness and strength, as well as regular structural behavior; this was demonstrated by fundamental vibration modes with stated fundamentals. The findings demonstrate the design process and the planned intervention effects to be successful. However, if you just use a tiny quantity of SPSW, you may have better retrofit design results. This means that a resistance-reducing device may be used to mitigate the impact of RC structures, which may result in no reinforcing of the structural part.

In future research, the emphasis will be on integrating appropriate design drawings for SPSWs into multi-layered RC structures for usage as a retrofit option for various kinds of existing structures. The diagrams shown here are valuable design tools for meeting the many performance-based design criteria, as well as the demands of current design approaches. This simple and successful passive control seismic system has been used to a multi-story RC structure.

Christoph Mahrenholtz et al. (2014) Observations on the aftershock damage of recent earthquakes reveal that many concrete buildings from the era of the Great Depression may be vulnerable to strong earthquakes. The best way to increase the structural strength and ductility of a structure is to connect a steel support to it. Steel beam supports are particularly ideal for prefabrication, and because of its low weight are often used in utility, passageway, and lamp opening applications. Concrete anchored to the structure using post-installed concrete anchors is normally used with steel brackets. Even yet, they have certain limitations. They require a lot of steel, and installation is more complicated. While steelless brackets seem to be appealing, steel brackets look to be undesirable. This research fastened the brace to the gusset using an anchor bracket linked to the concrete anchor by an anchor bracket connected with a concrete anchor. By increasing the usage of the structural system and anchors, they have enabled the anchors to be constructed with greater accuracy and cost-effectiveness. The added benefit provided by a buckling restraint bracket (BRB) is compared to traditional stents. See you later, alligator. Seismic testing was done on BRBs that were bolted to post-installation anchors to find out how well they strengthen reinforced concrete frames. A feasibility study found that the suggested design technique increased ductility and strength, while also maintaining seismic performance.

Fardis (2013) A procedure that instructs you on the procedures needed to build a link. Moving a few pins may create relative movement, which must be simulated by connecting the filler and boundary frame. If the number of pins used in the connection interface is greater, significant movement may occur, so please be sure to include the connection interface between the filler and boundary frame in your models. Videos, which captured the experiment and examination of the damage, were utilized to determine which damage states are feasible. The building has four distinct damage stages, from the rupture of the RC wall to the total collapse owing to instability. The median of the logarithmic distribution model is used to construct a link between the probabilities that a Sd value reaches or exceeds certain damage states. As the result of this analysis, inserting the RC fill wall provides the structure with significant ductility (mu2) prior to it collapsing, which is deemed a success of the retrofit procedure.

III. CONCLUSION AND FUTURE WORK

The structural modernisation in general has raised the building's resistance to seismic shaking, which may be thought of as the structural modernization of the buildings at this time to prevent collapse. With far more assurance, there is no design burden. This research illustrates how simple a homework assignment is. Analysis may be used to assess the current and changed systems' seismic resistance. linear and non-linear analyses are used while working on the overall structure as well as small sections of the structure. One of the most notable benefits of a nonlinear payment scheme is that you may make far more complex payment sequences. An opportunity to measure damage may be found in analyses that go beyond linear. No difficult work Useful information on what to anticipate from future building performance during an earthquake may be found in the analysis.

References

- [1] Bark, H., Markou, G., Mourlas, C., & Papadrakakis, M. (2016, June). Seismic assessment of a 5-storey retrofitted RC building. In *ECCOMAS Congress, VII European Congress on Computational Methods in Applied Sciences and Engineering, Crete Island, Greece* (pp. 5-10).
- [2] Chrysostomou, C. Z., Kyriakides, N., Kotronis, P., & Georgiou, E. (2016). Derivation of Fragility Curves for RC Frames Retrofitted with RC Infill Walls based on Full-Scale Pseudodynamic Testing Results. In *Proceedings ECCOMAS Congress* 2016 (pp. 5-10).
- [3] Dang, C. T., & Dinh, N. H. (2017). Experimental Study on Structural Performance of RC Exterior Beam-Column Joints Retrofitted by Steel Jacketing and Haunch Element under Cyclic Loading Simulating Earthquake Excitation. Advances in Civil Engineering, 2017.
- [4] Ercan, E., Arisoy, B., & Ertem, O. B. (2019). Experimental Assessment of RC Beam-Column Connections with Internal and External Strengthening Techniques. *Advances in Civil Engineering*, 2019.
- [5] Foraboschi, P. (2016). Versatility of steel in correcting construction deficiencies and in seismic retrofitting of RC buildings. *Journal of Building Engineering*, 8, 107-122.
- [6] Formisano, A., & Mazzolani, F. M. (2015). On the selection by MCDM methods of the optimal system for seismic retrofitting and vertical addition of existing buildings. *Computers & Structures*, 159, 1-13.
- [7] Formisano, A., & Sahoo, D. R. (2015). Steel shear panels as retrofitting system of existing multi-story RC buildings: Case studies. In *Advances in Structural Engineering* (pp. 495-512). Springer, New Delhi.

- [8] Hadi, M. N., & Tran, T. M. (2016). Seismic rehabilitation of reinforced concrete beam–column joints by bonding with concrete covers and wrapping with FRP composites. *Materials and Structures*, 49(1-2), 467-485.
- [9] Karthik, S., & Sundaravadivelu, K. (2017, July). Retrofitting of Reinforced Concrete Beams using Reactive Powder Concrete (RPC). In *IOP Conference Series: Earth and Environmental Science* (Vol. 80, No. 1, p. 012038). IOP Publishing.
- [10] Mahrenholtz, C., Lin, P. C., Wu, A. C., Tsai, K. C., Hwang, S. J., Lin, R. Y., & Bhayusukma, M. Y. (2015). Retrofit of reinforced concrete frames with buckling-restrained braces. *Earthquake Engineering & Structural Dynamics*, 44(1), 59-78.
- [11] Mazza, F., Mazza, M., & Vulcano, A. (2015). Displacement-based seismic design of hysteretic damped braces for retrofitting in-elevation irregular rc framed structures. *Soil Dynamics and Earthquake Engineering*, 69, 115-124.
- [12] Obaidat, Y. T., Abu-Farsakh, G. A., & Ashteyat, A. M. (2019). Retrofitting of partially damaged reinforced concrete beamcolumn joints using various plate-configurations of CFRP under cyclic loading. *Construction and Building Materials*, 198, 313-322.
- [13] Puppio, M. L., & Ferrini, M. (2019). Parametric analysis on external dissipative link system for seismic protection of low rise rc buildings. *Frattura ed Integrità Strutturale*, 13(48), 706-739.
- [14] Seki, M., Miyazaki, M., Tsuneki, Y., & Kataoka, K. (2000). A masonry school building retrofitted by base isolation technology. In *Proceedings of the 12th World Conference on Earthquake Engineering*.
- [15] Tarfan, S., Banazadeh, M., & Esteghamati, M. Z. (2019). Probabilistic seismic assessment of non-ductile RC buildings retrofitted using pre-tensioned aramid fiber reinforced polymer belts. *Composite Structures*, 208, 865-878.
- [16] Vitiello, U., Ciotta, V., Salzano, A., Asprone, D., Manfredi, G., & Cosenza, E. (2019). BIM-based approach for the costoptimization of seismic retrofit strategies on existing buildings. *Automation in Construction*, *98*, 90-101.

